

Investigation of the Use of GGBS to Mitigate ASR in Concrete using Volcanic Aggregates from Tsing Yi North

Annual Concrete Seminar 2022

LOW-CARBON CONCRETE ON THE MOVE

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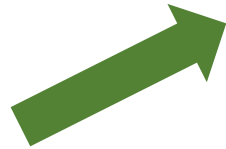
Geotechnical Engineering Office,

Civil Engineering and Development Department

10 November 2022

Introduction

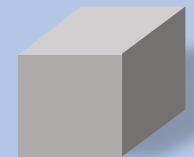
**Supplementary
cementitious
material (SCM)**



**Reduce cement
consumption to lower
carbon footprint**



**Mitigate the risk of
alkali-aggregate
reaction in concrete**

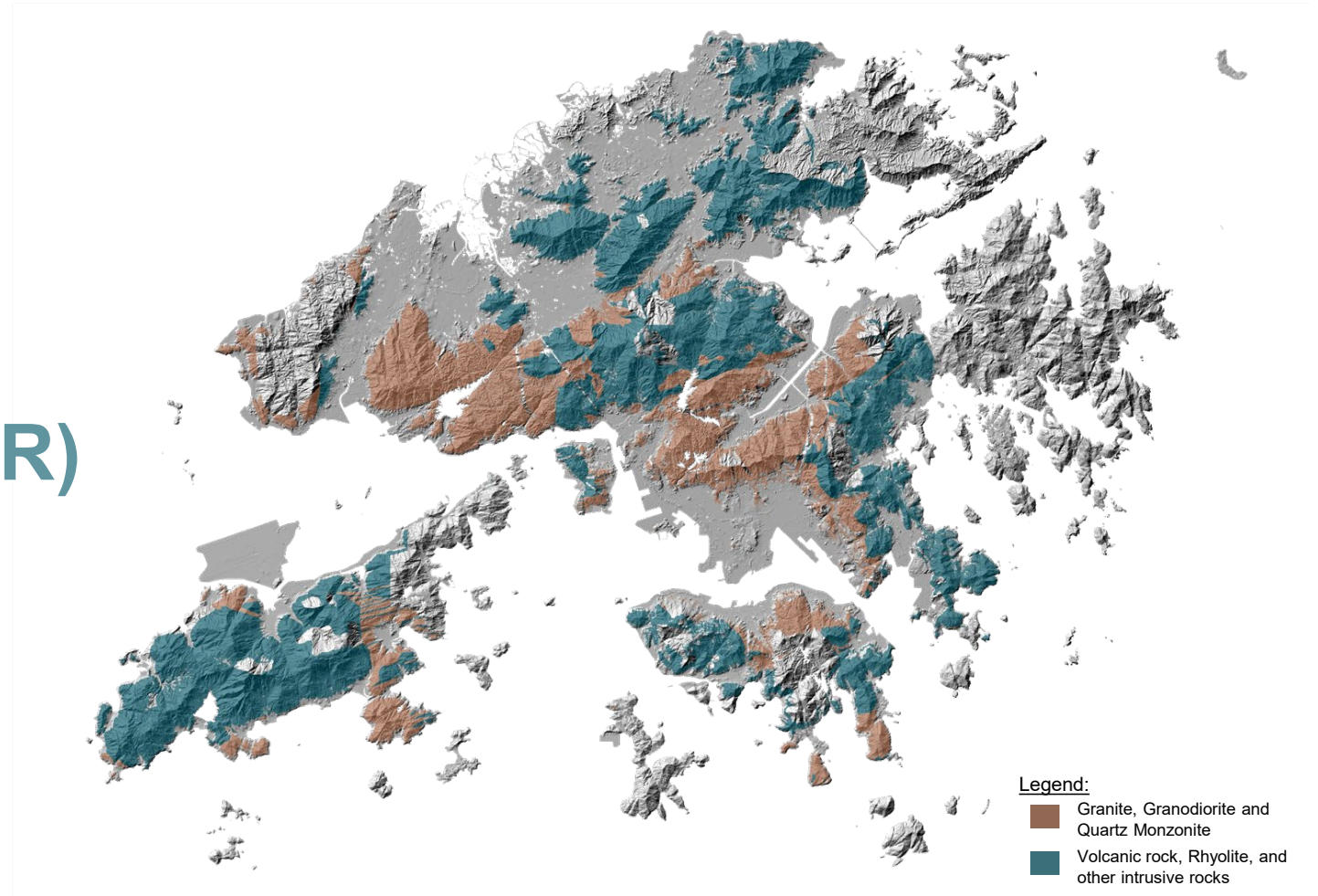


Choice of aggregate

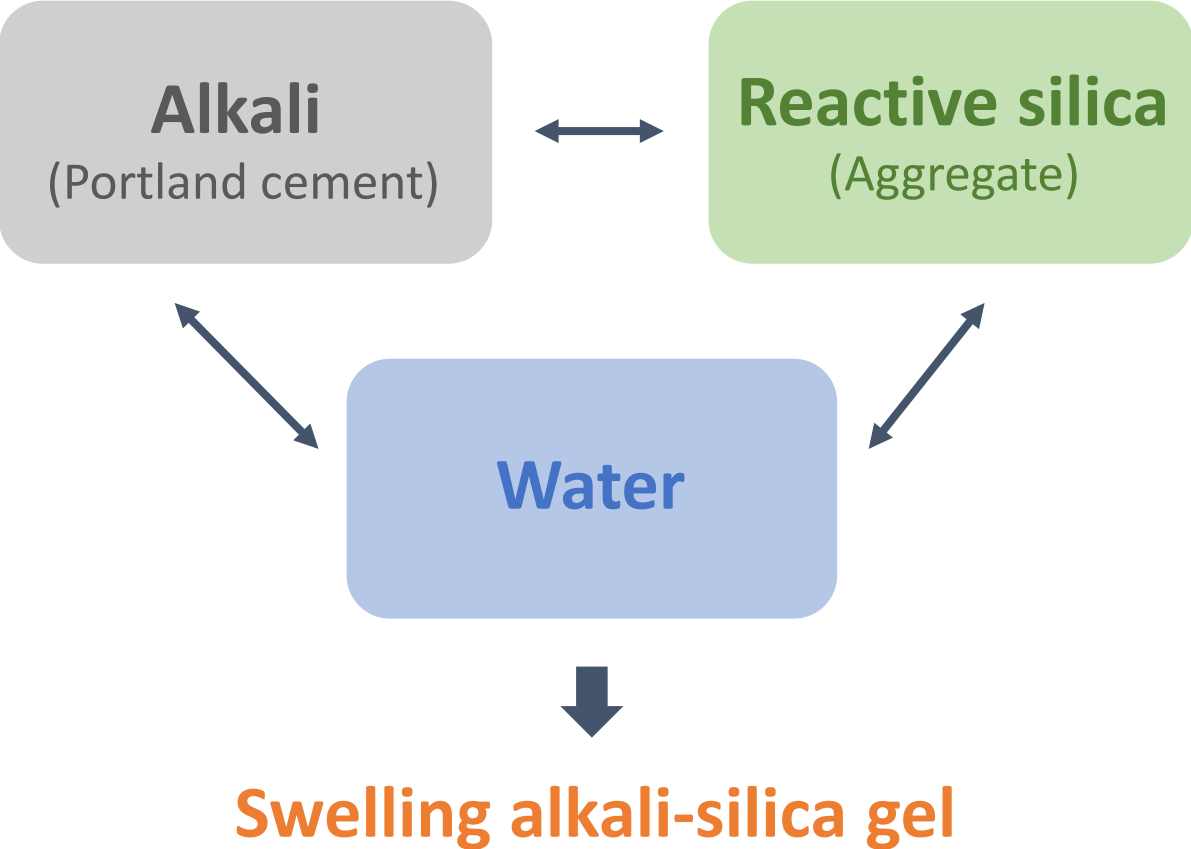
Volcanic rock and
rhyolitic dyke rock **50%**

Seldom used as aggregates for concrete
production due to the potential of
alkali-silica reaction (ASR)

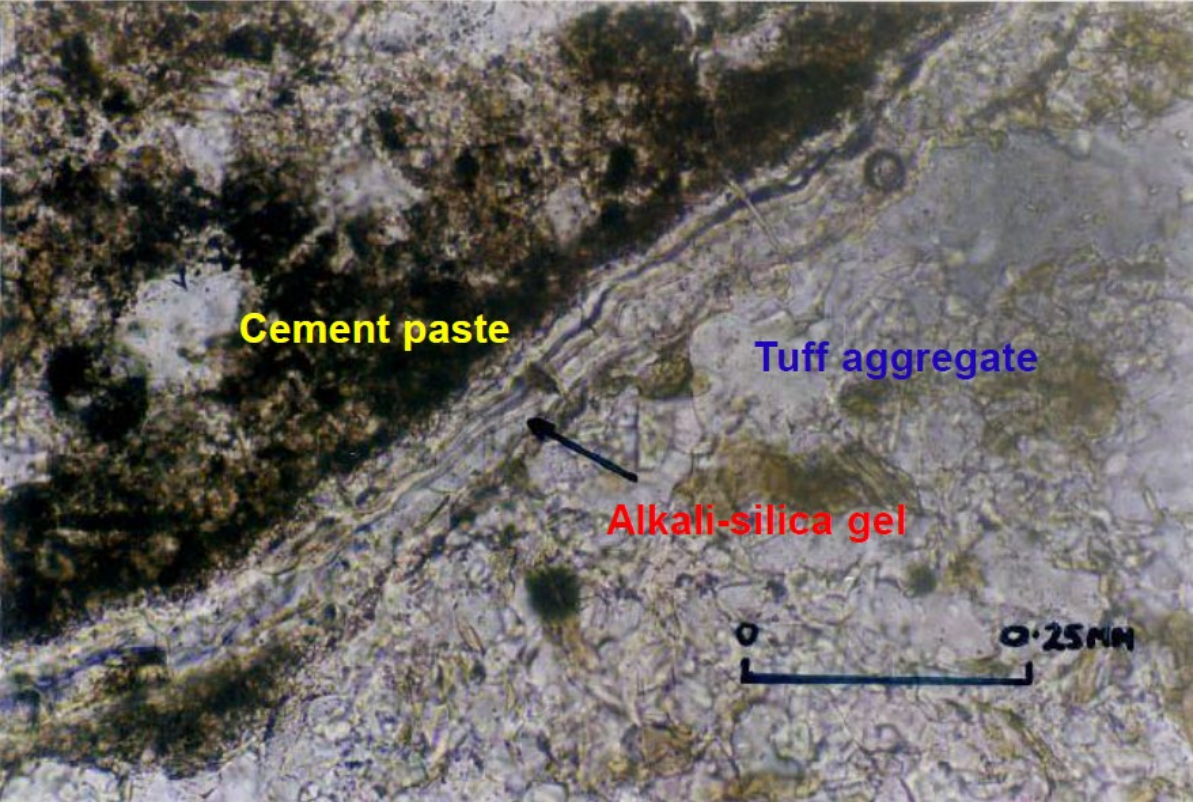
Very limited choice of
quarrying site



What is ASR?



Cracking in the long term



Cases of ASR in Hong Kong

Site	Year Constructed	Year ASR Reported	Approximate Time Lapsed
Shek Wu Hui Treatment Works	1980 – 1983	1991	10 years
Fanling foot bridge	1982	1998	16 years
North Point Government School	1986 – 1988	1999	12 years
Hill Road Flyover	Completed in 1982	1997	15 years
CLP Tseng Tsui Ash Lagoon	1988	1999	11 years



Projects	Aggregates		Remedial Works
Shek Wu Hui Sewage Treatment Works (Year Constructed: 1983)	<ul style="list-style-type: none"> ▪ Volcanic ▪ Wu Shek Ku Quarry, Shenzhen ▪ AMBT 0.309% 	Metatuff, foliated, altered abundant finely recrystallized quartz & strained quartz crystal	Surface cleaning, Rendering mortar with latex additive



Shek Wu Hui Treatment Plant, cracks identified in 1991

Prevention of Alkali-Silica Reaction (ASR)

Three basic requirements for ASR to occur in concrete:

- Sufficient quantity of reactive silica (in aggregates)
- Sufficient concentration of alkali (primarily from cement)
- Sufficient supply of water (in hardened concrete, e.g. moisture)

Guidelines in Australia, Canada, France, Germany, Japan, New Zealand, RILEM, UK

ASR Preventive Method Includes:

- Avoid the use of reactive aggregates
- Use of non-reactive aggregate combination
- Limit alkalis content of concrete
- Use supplementary cementing materials (SCMs), e.g. PFA, GGBS
- Use of suitable chemical admixtures
- No water contract or reducing relative humidity in hardened concrete

Prevention of Alkali-Silica Reaction (ASR)

Project Administrative Handbook for Civil Engineering Works and General Specification:

Reduction of the amount of alkalis present (expressed as equivalent sodium oxide) in the concrete to 3kg/m^3 .

1. the reduction of amount of cement used in the concrete mix,
2. the use of low alkali cement, or
3. **the use of supplementary cementitious materials (SCM), such as PFA and GGBS**

For structural elements for which ingress of moisture of the concrete is not possible throughout the design life, the 3kg/m^3 limit on alkali reactive content need not apply.

Buildings Department - PNAP APP-74

1. Equivalent sodium oxide $< 3\text{kg/m}^3$
2. Unless concrete element will not be subject to ingress of moisture throughout its design life

Rare application in Hong Kong

Projects	Aggregates / Concrete	Measures
International Commerce Centre (Year Constructed: 2010)	Volcanic aggregates (Anderson Road)	<ul style="list-style-type: none">▪ Permanent waterproofing plaster for critical locations▪ PFA 35% and Silica Fume (6%)▪ 3kg/m³ alkaline control▪ W/C Ratio 0.28
	Grade 90 High Elastic Modulus Concrete Test results: 100~120MPa, E.M.: 39-50GPa	



International Commerce Centre

Supplementary Cementitious Materials (SCM)

Why SCM is useful in mitigating ASR?

- PFA – Pulverized Fuel Ash
- GGBS – Ground Granulated Blastfurnace Slag
- SF – Silica Fume

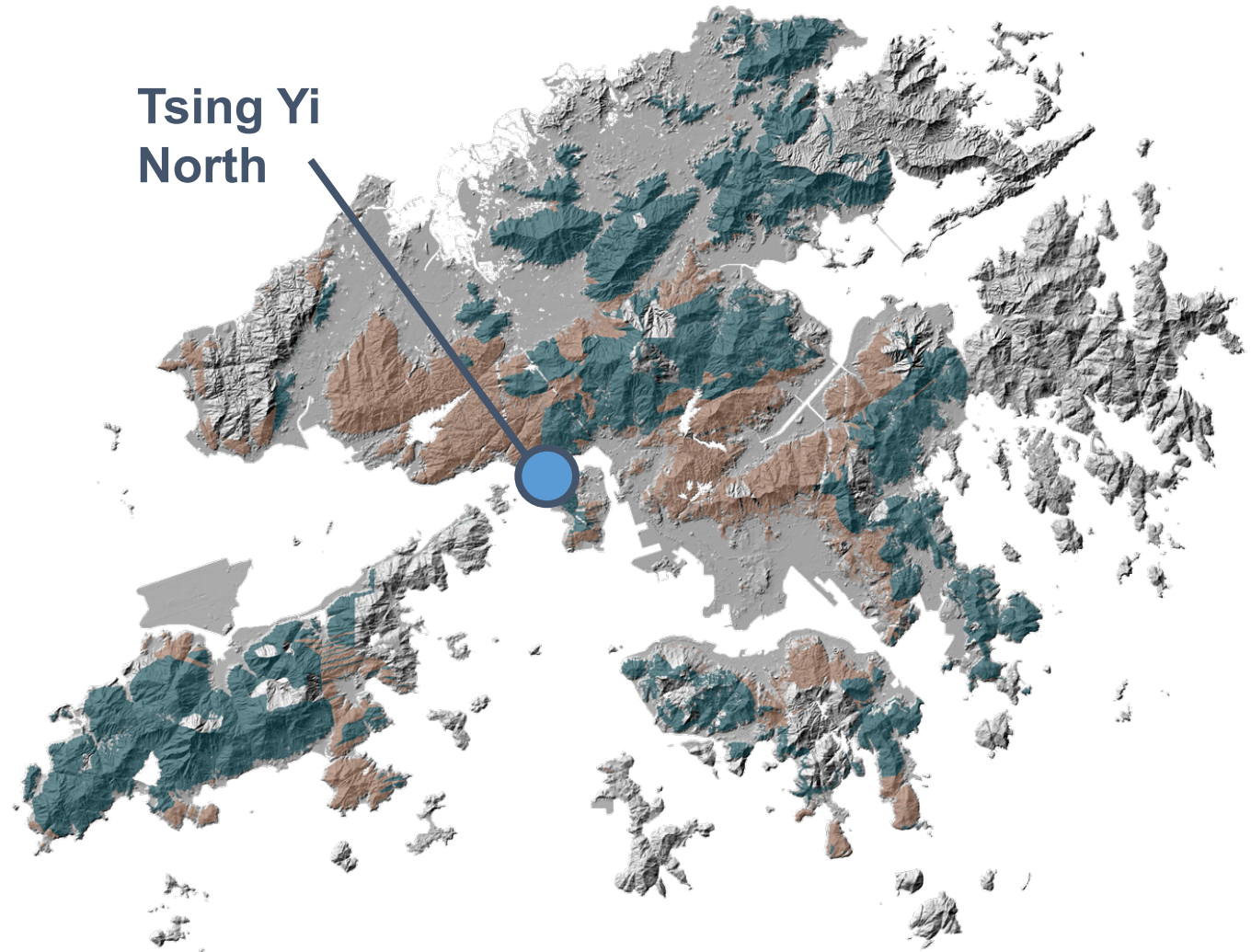
Alkalis content
are rapidly consumed

Reduce permeability
of concrete

Site Specific Tests

To verify the use of SCM in local **volcanic and rhyolitic** dyke rock to suppress ASR

To find out **range of mixes** with SCM to suppress ASR for further investigation



Testing programme

With reference to RILEM's assessment framework:

Petrographic Examination on thin section

- CS3:2013 Section 9
- Look for aggregates with potential deleterious constituents

Rapid Screening

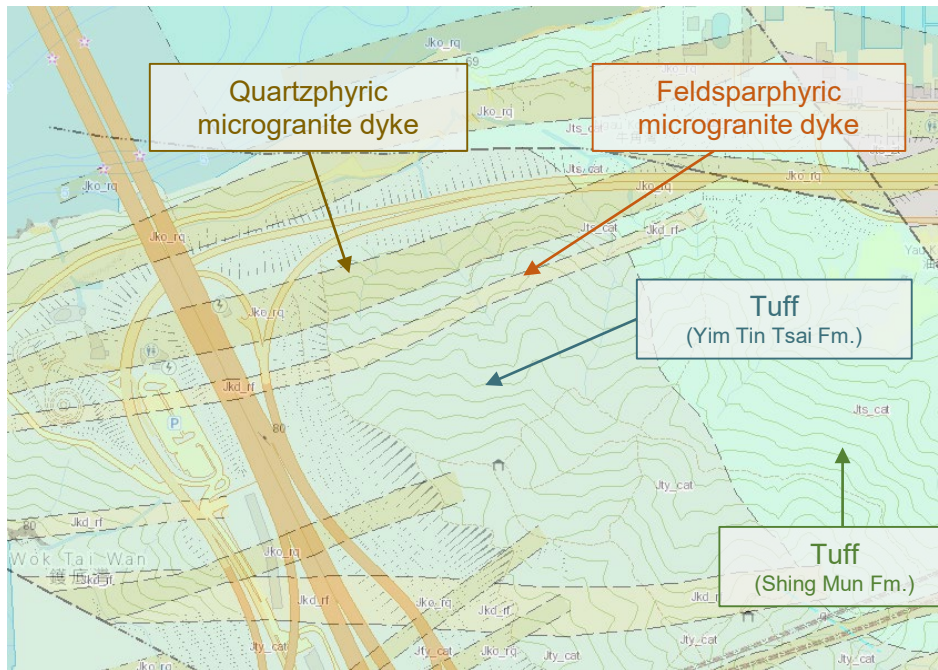
- 14-day Ultra-accelerated Mortar Bar Test (UAMBT)
- CS1:2010 Section 22
- To provide early indication of ASR potential

Concrete Expansion Testing

- 52-week Concrete Prism Test (CPT)
- CS1:2010 Section 23 with modification to add SCM
- Reference test to give confirmative results on ASR Potential

Geology at Tsing Yi North

- Tuff (predominant)
- Feldsparphyric microgranite dyke
- Quartzphyric microgranite dyke



Geology Map (1:20,000)

Tuff:

Strong, dark grey, mottled grey and white, slightly decomposed coarse ask crystal TUFF. No apparent joint.

Feldsparphyric microgranite dyke:

Strong, pinkish grey to light pinkish grey, spotted blacked, mottled pink and white, slightly decomposed FELDSPARPHYRIC MICROGRANITE. No apparent joint.

Quartzphyric microgranite dyke:

Moderately strong to strong, light brown, spotted grey and white moderately to slightly decomposed QUARTZPHYRIC MICROGRANITE. Joints are medium to closely spaced, smooth planner, extremely narrow, stained with iron and manganese oxide.



Tuff



Feldsparphyric
Microgranite dyke

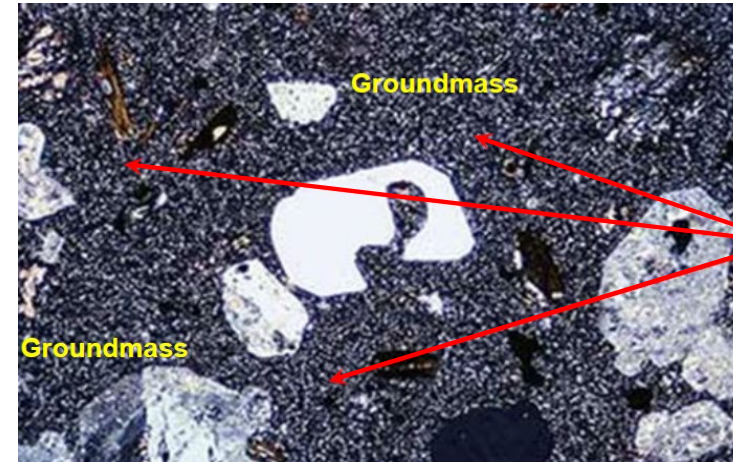


Quartzphyric
microgranite dyke

Petrographical examinations

For aggregates (Section 9 of CS3:2013)

- The groundmass was determined based on point counting analysis by traversing in regular increments over the thin section in the form of a grid pattern.

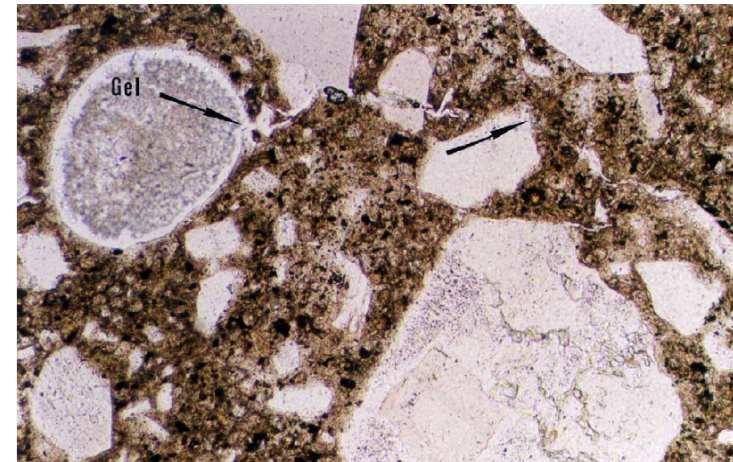


Microcrystalline to cryptocrystalline quartz

Thin section of aggregate

For concrete (CS1:2010)

- Concrete prism of expansion beyond 0.05% in the Concrete Prism Test were taken for further petrographic examination



Thin section of concrete prism

UAMBT and CPT

UAMBT - Section 22 of CS1:2010

- Determine rapidly (**14 days**) the potential alkali-reactivity of aggregates
- Measurement of the expansion of mortar-bars immersed in NaOH solution at elevated temperature.



Expansion After 14 Days of Immersion in NaOH solution (%)	Potential Reactivity
< 0.10	Non-reactive
0.10 to 0.20	Potentially reactive
> 0.20	Reactive

CPT - Section 23 of CS1:2010 with modification

- SCM is added in the concrete prism
- Measurement of expansion (**52 weeks**) of concrete produced by alkali-silica reaction.



Expansion After 52 Weeks (%)	Potential Reactivity
< 0.05	Non-reactive
0.05 to 0.10	Potentially reactive
> 0.10	Reactive

Mixes adopted in the tests

Summary of CPT results in GEO Report No. 354

Cement replacement level	Average Expansion in 52-week (%)	
	ARQ	Lam Tei
0% (Norcem only)	0.11 (Reactive)	0.12 (Reactive)
35% PFA	0.00	0.00
30% PFA + 5% CSF	0.00	-0.01
50% GGBS	0.01	0.00
70% GGBS	0.01	0.00
50% GGBS + 5% CSF	0.00	0.00

Mix adopted in this study

No.	Cement replacement level
1	0% (Norcem only)
2	25% PFA
3	35% PFA
4	30% PFA + 5% CSF
5	50% GGBS
6	70% GGBS
7	50% GGBS + 5% CSF
8	0 % (ASR Inhibitor)

To enable direct comparison, the adopted cement replacement levels are same as that in the GEO Report No. 354 with 2 additional mixes

Other tests

Index Test

CS3:2013 – Aggregates for Concrete

Physical tests

- Flakiness Index
- Elongation Index
- Los Angeles Value
- Mean Aggregate Impact Value
- Mean Ten Percent Fines Value
- Oven-dried Particle Density
- Water Absorption
- Magnesium Sulphate Soundness
- Drying Shrinkage

Chemical tests

- Water-soluble Chloride Ion Content
- Acid-soluble Sulphate Content



Overall testing schedule

Index Tests, Petrographic Examination (aggregate), UAMBT

- Tuff
- Feldsparphyric microgranite dyke
- Quartzphyric microgranite dyke

CPT

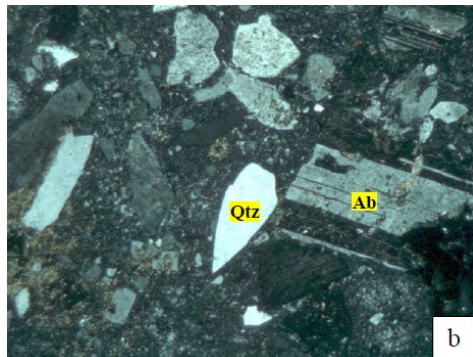
No.	Cement replacement level	Tuff	Feldsparphyric microgranite dyke	Quartzphyric microgranite dyke
1	0% (Norcem only)	✓	✓	✓
2	25% PFA	✓	✓	✓
3	35% PFA	✓	✓	✓
4	30% PFA + 5% CSF	✓	✓	✓
5	50% GGBS	✓	✓	✓
6	70% GGBS	✓	✓	✓
7	50% GGBS + 5% CSF	✓	✓	✓
8	0 % (ASR Inhibitor)	✓	✓	✓

Petrographic Examination (concrete prism)

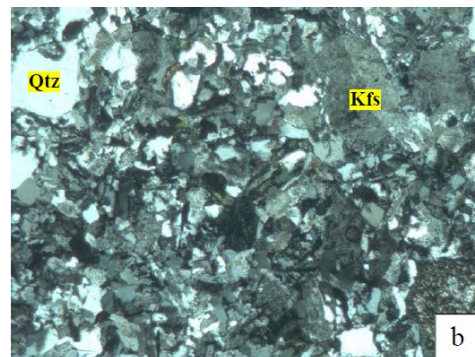
- Specimens with expansion >0.05%

Petrographic Examination on Aggregate

Petrographic examination	Tuff	Feldsparphyric microgranite dyke	Quartzphyric microgranite dyke
Classification of samples	Potentially reactive attributed to strained quartz crystal fragments and cryptocrystalline to microcrystalline quartz	Potentially reactive attributed to strained quartz phenocrysts and quartz of the fine-grained groundmass	Potentially reactive attributed to strained quartz phenocrysts and quartz of the fine-grained groundmass



Tuff



Feldsparphyric
Microgranite Dyke



Quartzphyric
Microgranite Dyke



UAMBT and CPT result (without SCM)

Aggregate	UAMBT		CPT	
	14-Day Expansion (%)	ASR Potential	52-Week Expansion (%)	ASR Potential
Tuff	0.24	Reactive	0.075	Potentially reactive
Feldsparphyric microgranite dyke	0.14	Potentially reactive	0.1	Potentially reactive
Quartzphyric microgranite dyke	0.15	Potentially reactive	0.03	Non-reactive

Expansion After 14 Days of Immersion in NaOH solution (%)	Potential Reactivity
< 0.10	Non-reactive
0.10 to 0.20	Potentially reactive
> 0.20	Reactive

Expansion After 52 Weeks (%)	Potential Reactivity
< 0.05	Non-reactive
0.05 to 0.10	Potentially reactive
> 0.10	Reactive

Summary of CPT Results (52 weeks)

Cement replacement level	Tuff	Feldsparphyric microgranite dyke	Quartzphyric microgranite dyke
0% (Norcem only)	0.075%	0.1%	0.030%
25% PFA	0.005%	0.025%	0.025%
35% PFA	-0.005%	0.025%	0.025%
30% PFA + 5% CSF	0.000%	0.025%	0.025%
50% GGBS	0.010%	0.035%	0.035%
70% GGBS	0.005%	0.030%	0.035%
50% GGBS + 5% CSF	0.000%	0.030%	0.035%
0 % (ASR Inhibitor)	0.010%	0.045%	0.035%

Observations

1. With SCMs, all expansion is less than 0.05%
2. No significant benefit for higher dosage of 35% PFA than 25% PFA.
3. Higher dosage of 70% GGBS shows slightly better results than 50% GGBS
4. Reactiveness of feldsparphyric microgranite is higher than tuff before and after treated
5. CSF has no significant effect in general
6. ASR inhibitor is less effective compared to SCMs

Expansion After 52 Weeks (%)	Potential Reactivity
< 0.05	Non-reactive
0.05 to 0.10	Potentially reactive
> 0.10	Reactive

Summary of CPT Results (52 weeks)

Cement replacement level	Tuff ¹ (Anderson Road)	Tuff ¹ (Lam Tei)	Tuff (Tsing Yi North)
0% (Norcem only)	0.11%	0.12%	0.075%
25% PFA	-	-	0.005%
35% PFA	0.00%	0.00%	-0.005%
30% PFA + 5% CSF	0.00%	-0.01%	0.000%
50% GGBS	0.01%	0.00%	0.010%
70% GGBS	0.01%	0.00%	0.005%
50% GGBS + 5% CSF	0.00%	0.00%	0.000%
0 % (ASR Inhibitor)	-	-	0.010%

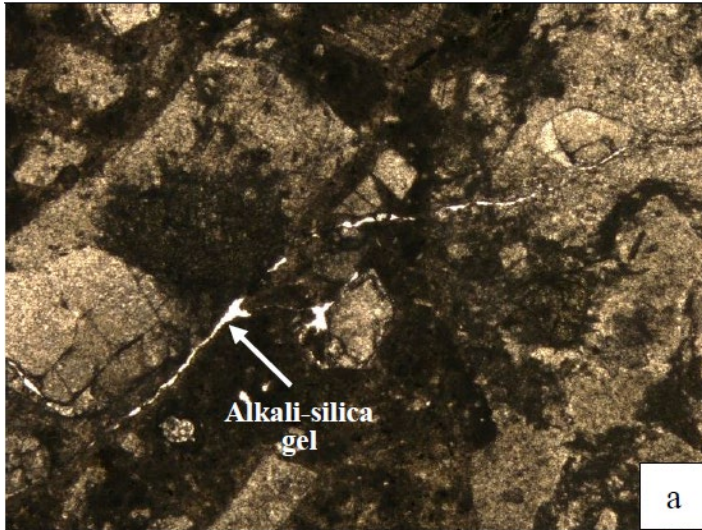
¹ Reference: GEO Report No. 354

Observations

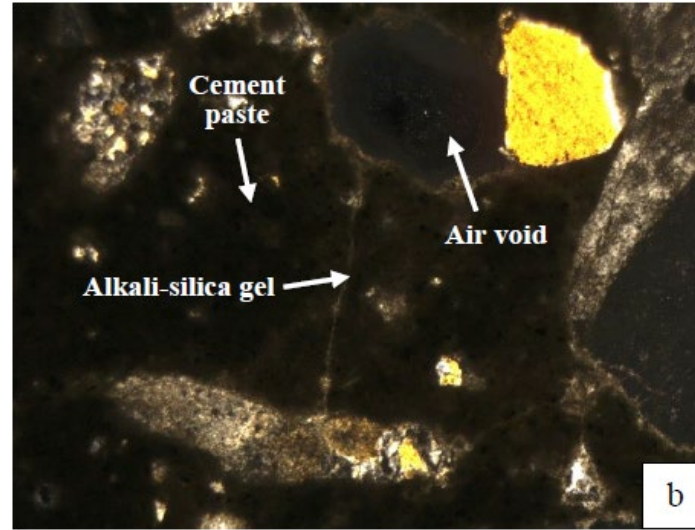
1. The tuff at Tsing Yi is less reactive than those in Anderson Road and Lam Tei
2. With SCMs, all expansion is less than 0.05%
3. The increase of effectiveness of SCM is not significant when it is beyond certain replacement level
4. CSF has no significant effect in general
5. The result is very consistent

Expansion After 52 Weeks (%)	Potential Reactivity
< 0.05	Non-reactive
0.05 to 0.10	Potentially reactive
> 0.10	Reactive

Petrographical examination on concrete prism

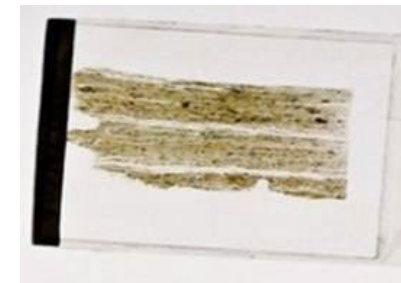


Concrete prism using
Tuff as aggregate



Concrete prism using
Feldsparphyric Microgranite
dyke as aggregate

- Concrete prisms with expansion beyond 0.05% in the CPT were cut into thin section
- Alkali-silica reaction is the likely cause of the expansion



Conclusion

- a) A complete set of tests, include index tests, petrographical examination, UAMBT and CPT according to RILEM's assessment framework and CS1 : 2020 were carried out.
- b) The test results indicate that the application of the SCMs is effective in suppressing the potential deleterious effect for volcanic tuff and rhyolitic dyke rock in the Tsing Yi North.
- c) The increase of effectiveness of SCM is not significant when it is beyond certain replacement level.
 - No significant benefit for higher dosage of 35% PFA than 25% PFA.
 - Higher dosage of 70% GGBS shows slightly better results than 50% GGBS.
- d) CSF has no significant effect in general.